CAPACITY OF TRAFFIC SIGN CONTROLLED JUNCTION

- based on the number of vehicles which can drive through a junction at a certain time interval
- determined by calculating capacity of minor traffic flows and the resulting traffic delays at the minor roads
- Iimited by capacity in every collision point of the junction where traffic flows connect, disconnect or intersect fig. 0430

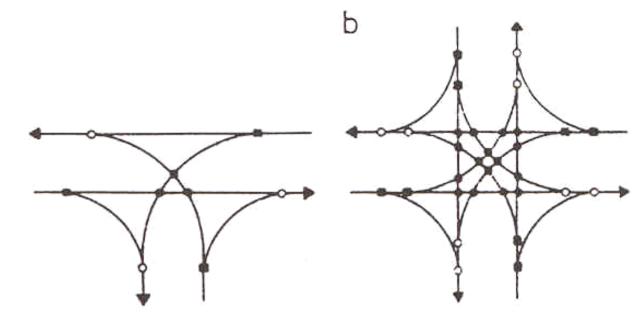


fig. 0430 (collision points at junctions)

a) **T-junction** – 3 legs (entries/approaches)

a

- b) crossroads 4 legs (entries/approaches)
- the unfavourable combination of traffic flows is critical for total junction performance (labelling in *fig. 0440*)

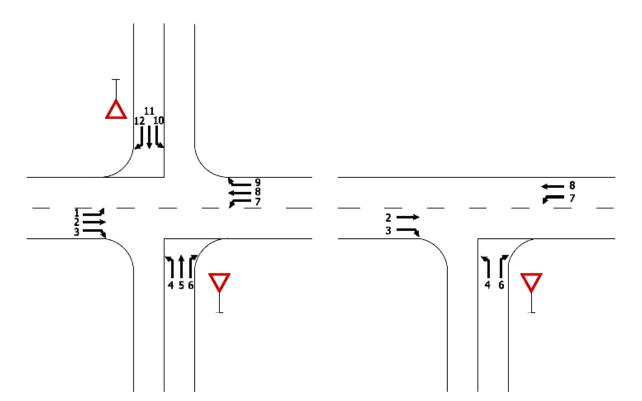


fig. 0440 (labelling of traffic flows in a junction)

We consider T-junction in our case

<u>Diagram of traffic flows</u> in *fig.* $0450 \Rightarrow$ stages ("k"):

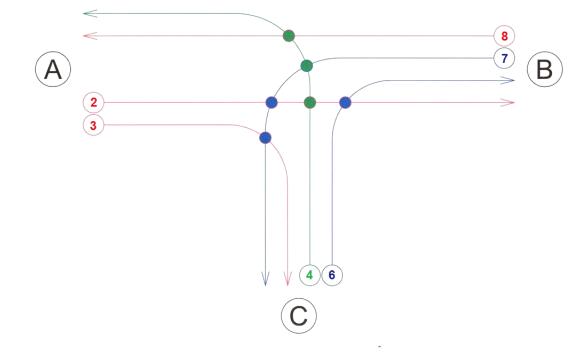


fig. 0450 (diagram of traffic flows of T-junction)

- 1. stage flows 2, 8, 3
- 2. stage flows 6, 7 (first inferiority)
- 3. stage flow 4 (second inferiority)

Determining the junction model:

 sets solution of right turn (corner or a slip road – *fig. 0460*) on the basis of classes of the crossing roads

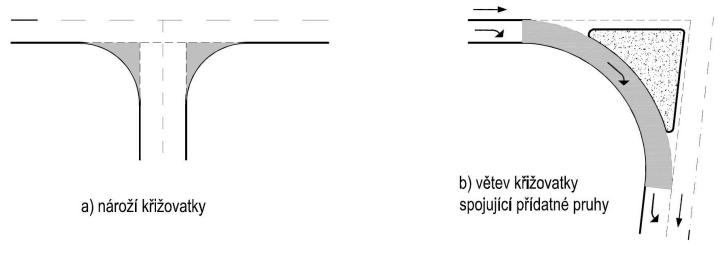


fig. 0460 (corner and slip road of at-grade junction)

 determine the class of both roads and then the junction model ("SÚK") according to the following table (the table is based on preliminary determination of roads category in ČSN 736101 – simplified and adapted for the needs of the exercise)

Determining the junction model of at-grade T-junction ("SÚK")		Minor road		
		S 6,5	S 7,5	
		Third- class	Second class	
Major road	S 7,5	Second- class	SÚK IV	SÚK V
	S 9,5	Second- class	SÚK III	SÚK V
	S 11,5	First- class	SÚK III	SÚK VI

<u>Junction model</u> (defined in table in *fig. 0470* – mention in the technical report !!!) **provides:**

- solution of the right turn
- existence of the auxiliary turning lanes (simplified for needs of the exercise)

Označení křižovatky	Cabárra huru	Usměrnění o	Třídy křižijících		
	Schéma typu	hlavní silnici H ^{a)}	vedlejší silnici V ^{b)}	se silnic H/V	
SÚK I	H 			a) III tř. / III tř. b) II tř. / III tř.°)	
SÚK II	H V		dělicí ostrůvek	a) tř. / tř. b) tř. / tř. c) tř. / tř. °)	
SÚK III	H V \ I		dělicí a směrovací ostrůvek	a) tř. / tř. b) tř. / tř. °)	
SÚK IV	H 2 5,50m (5,0m cl) V	rozšíření zpevněné krajnice pro objíždění vozidel odbočujících vlevo		a) tř. / tř. b) tř. / tř. c) tř / tř. °)	
súk v		dělicí ostrůvky a odbočovací pruhy pro levé odbočení	dělicí a směrovací ostrůvek	a) tř. / tř. b) tř. / tř.	
súk vi		dělicí ostrůvky, odbočovací pruhy a připojovací pruhy	dělicí a směrovací ostrůvky	a) tř. / tř. b) tř. / tř.	

c Při nízké intenzitě dopravy

fig. 0470 (recommended models of at-grade T-junction on two-lane roads)

Draw the diagram of junction lanes arrangement (according to the example in *fig. 0480*):

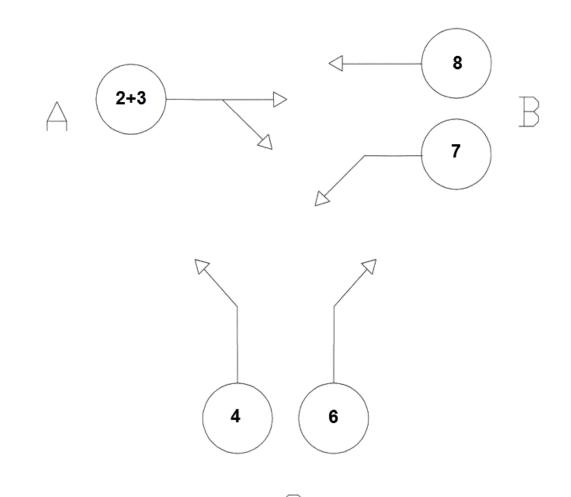


fig. 0480 (example of diagram of junction lanes arrangement)

<u>Draw the traffic load (volume) diagram</u> – scale 1 mm \approx 20 vehicles/hour (according to *fig. 0490*):

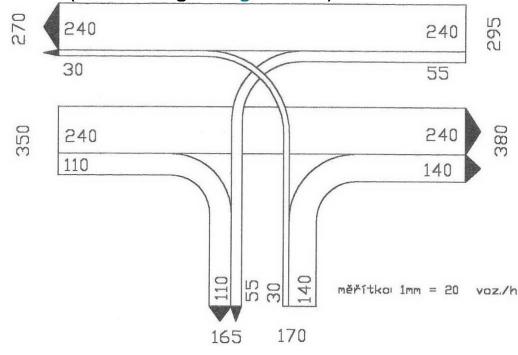


fig. 0490 (an example – the traffic load diagram)

<u>Input</u> – real vehicles + percentage of slow vehicles (on major and minor roads) \Rightarrow convert to so-called "pveh/h" (converted vehicles / hour) according to the formula:

 $I_{i}[pveh/h] = I_{i}[veh/h] \bullet \left(\frac{100\% - ppv[\%]}{100\%} + \frac{k_{pv} \bullet ppv[\%]}{100\%}\right)$

- consider "ppv" according to the road (major / minor) from where the traffic flow exits
- coefficient $_{k_{pv}}$ is generally obtained from the table in fig. 0500 \implies use value $k_{pv} = 1,5$ for the purpose of exercise (we consider lorries and busses only)

Typ křižovatky	Jízdní kola	Motocykly	Osobní vozidla ^ª	Nákladní vozidla, autobusy ^b	Nákladní soupravy, kloubové autobusy
Průsečné a stykové bez SSZ	0,5	0,8	1,0	1,5	2,0
Průsečné a stykové se SSZ	0,5	0,8	1,0	1,7	2,2
Okružní	0,5	0,8	1,0	2,0	3,0

^a Včetně nákladních vozidel do 3,5 t celkové hmotnosti.

^b Nákladní vozidla nad 3,5 t celkové hmotnosti mimo nákladních souprav a autobusy mimo kloubové autobusy.

fig. 0500 (recommended converting coefficients of the traffic mixes)

The capacity calculation is based on:

the number of space-time gaps between vehicles of traffic flow with the right of way priority (superior flow "j") which are acceptable for merging or crossing with vehicles of inferior traffic flow so their manoeuvres are smooth, safe and without loss of time ⇒ percentage (P^{k(j)}) of time of

free traffic flow ",j" of stage ",k", if the flow ",j" is inferior to the other traffic flows of stage ",k-1" at the same time

- determining the critical time headway ",tg" between vehicles of superior traffic flow ",i" (the first waiting vehicle)
- determining the mean time headway "tf" between vehicles of superior traffic flow "i" (the second and another waiting vehicles)

The calculation is based on the following values:

- I_i [pveh/h] design (outlook) traffic volumes of traffic
 - flows "i" (from the input values "I_i" [veh/h] a "ppv" [%])
- t_{g,i} [s] critical vehicle time headway of traffic flows "i" (see the table)

Value "t _g " According to the type of junction movement and speed of vehicles on the major road "v _h " [km/h]				
Type of traffic flow ("i")		t _g [s]		
Left turn from major road	7	3,4 + (0,021 ● v _h)		
Right turn from minor road	6	2,8 + (0,038 ● v _h)		
Left turn from minor road	4	5,2 + (0,022 ● v _h)		

- set "t_g" for each of traffic flows 4, 6 and 7
- "v_h" = 90 km/h (rural area)
- t_{f,i} [s]..... average vehicle time headway of traffic flows "i" (see the table)

Values t _f according to the type of junctio	n movement
and type of giving way to vehicles on th	e major road

Type of traffic flow ("i")		P6 🚥	P4 🔽
		t _f [s]	
Left turn from major road	7	2,6	
Right turn from minor road	6	3,7 3,1	
Left turn from minor road	4	4,1	3,5

set "t_f" for each of traffic flows 4, 6 and 7

I_{Hi} [veh/h] sums of traffic flow volumes superior to flows "i" (calculate according to the table , take the existence of off lanes into account)

Calculation of I _{Hi}					
Inferior flow "i" IHi [veh/h]					
Left turn from major road	7	₂ + ₃			
Right turn from minor road	6	I ₂ + 0,5 ● I ₃ ¹⁾			
Left turn from minor road	4	$ _2 + _8 + _7 + 0,5 \bullet _3^{(1)}$			
¹⁾ separate diverging lane for flow 3 (right turn from major road) \Rightarrow I ₃ = 0					

- G_i [pveh/h] basic flow capacity
- C_i [pveh/h] capacity of flow "i"
- Rez_i [pveh/h].... performance reserve of flow "i"
- t_{w,i} [s]..... mean time loss of flow "i"
- p_{0,7} [-]..... probability of free flow of traffic flow 7 (left turn from major road)

DETERMINING THE TRAFFIC FLOW CAPACITY Basic capacity "G_i" of flow "i":

$$G_{i} = \frac{3600 \bullet e^{-\frac{I_{H,i}}{3600} \bullet \left(t_{g,i} - \frac{t_{f,i}}{2}\right)}}{t_{f,i}}$$

$$[G_{i}] = pveh/h \qquad [I_{H,i}] = veh/h$$
Determining capacity "C_i" of flow "i":

- stage one flows (2, 3, 8) do not assess (they are not delayed)
- 2. <u>stage two flows (6, 7) first inferiority</u> $C_{6,sam} = G_6$ $C_7 = G_7$
- 3. <u>Stage three flows (4) second inferiority:</u>

$$p_{0,7} = 1 - \frac{I_7}{C_7}$$

$$[I_7] = [C_7] = pveh/h$$

$$C_{4,sam} = p_{0,7} \bullet G_4 \Longrightarrow C_{4,sam} = G_4 \bullet \left(1 - \frac{I_7}{C_7}\right)$$

4. <u>Consideration of impact of the common lane for two</u> inferior flows (4, 6):

$$A_{4+6} = \frac{I_4}{C_{4,sam}} + \frac{I_6}{C_{6,sam}}$$

[I_4] = [I_6] = [C_{4,sam}] = [C_{6,sam}] = [C_4] = [C_6] = pveh/h
$$C_4 = I_4 \bullet \left(\frac{1-A_{4+6}}{A_{4+6}}\right)$$
$$C_6 = I_6 \bullet \left(\frac{1-A_{4+6}}{A_{4+6}}\right)$$

DETERMINING PERFORMACE RESERVE AND TIME LOSS

Determine the **performance reserve "Rez**i" for each traffic flow "i" of stage two and higher:

$$\operatorname{\mathsf{Re}}_{i} = \operatorname{\mathsf{C}}_{i} - \operatorname{\mathsf{I}}_{i}$$

$[Rez_i] = [C_i] = [I_i] = pveh/h$

Determine mean time loss "tw,i according to the knowledge of capacity reserve "Rezi" according to the graph in *fig. 0510* (deduct the dependency of "Rezi" and "tw,i" according to the curve that corresponds as close as possible to the flow capacity "Ci"):

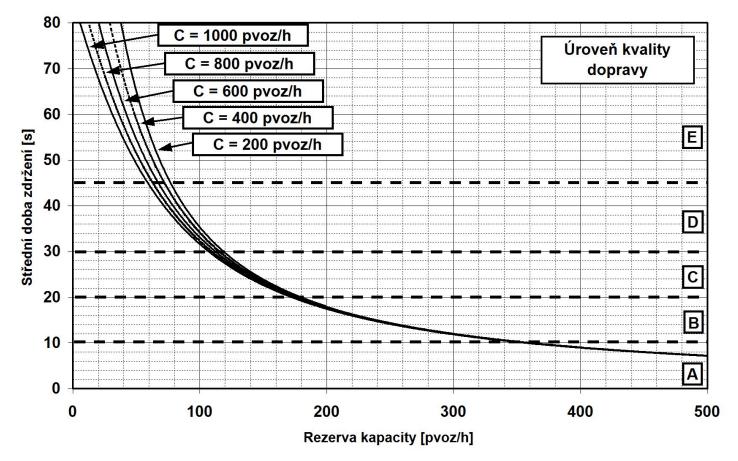


fig. 0510 (graph for determining the mean time loss)

FINAL ASSESSMENT AND RESULTS ARRANGEMENT

Determine the level of service "LOS" for each traffic flow of stage two and more according to the knowledge of mean time loss "t_{w,i}":

	LOS	t _{w,i}
label	Verbal description	[S]
А	Time loss is very small	0 – 10
В	Delay is without queues	10 – 20
С	Sporadic short queues, noticeable time loss	20 – 30
D	Stable state with high losses \Rightarrow vehicle queues	30 – 45
E	Unstable state (queue), time loss is not decreasing, sensitive behavior of dependency of traffic loads and losses	> 45
F	Capacity is exceeded, overloaded junction, vehicle queue grows	Rez _i < 0

ASSESSMENT

- standard ČSN 73 6102 requires following LOS_p:
 - $S I \dots LOS_p = C$
 - *S II*..... *LOS*_p = *D*
 - *S III*.....*LOS*_p = *E*

✤ For the purpose of the exercise ⇒ the junction meets capacity requirements if LOS ≤ LOS_p for each traffic flow according to the road class

Arrange the result into the clear table:						
Flow (i)	2 + 8	3	7	6	4	
Stage (k)	1	1	2	2	3	
INPUT AND	WORK	(ING V/	ALUES			
l _i [veh/h]						
l _i [pveh/h]						
t g,i [S]						
t _{f,i} [S]						
I _{н,i} [veh/h]						
p _{0,i} [—]						
DETERMINIG THE CA	PACITY	, RESE	RVES	, AND I	LOS	
G i [pveh/h]						
C i [pveh/h]						
Rez i [pveh/h]						
t _{w,i} [S]						
LOS _{p,i} [A/B/C/D/E]						
LOS _i [A/B/C/D/E/F]						
Meets capacity						
requirements [Y/N]						

- Write the complete calculation and verbal conclusion into • the technical report
- Pay attention to units !!! *